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1 Characterization and modelling of a carbon ramming  
2 mix used in high-temperature industry

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7 **Abstract**

This paper is devoted to the modelling of a specific ramming mix mainly used in the high-temperature industry due to its high-compacting behaviour. This material has the ability to absorb the deformation of parts submitted to high thermal loads. Triaxial tests were carried out in order to identify the shear behaviour and an instrumented die compaction test was developed for the hardening behaviour identification. Tests on the ramming mix were lead for a temperature range between 20°C and 80°C. The temperature effect is particularly observed on the material response when compacted. The main features of the behaviour of the ramming mix can be represented by the theoretical framework of the Modified Cam-Clay model. A single variable allows to accurately reproduce the hardening behaviour depending on temperature. Moreover an extension of the model for the hardening behaviour at high pressures is proposed. A good agreement between the experimental data and numerical tests is reached with this model.

8 *Keywords:* ramming mix, parameters identification, die compaction test,  
9 triaxial test, temperature effect, finite element, Modified Cam-Clay model

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## 10 **1. Introduction**

11 The steel industry requires huge structures mainly composed of refractory  
12 materials. The thermomechanical properties of these materials perfectly suit  
13 high temperature applications. However, the expansion of these constituents  
14 can damage some parts of the structure. To avoid this damage, ramming  
15 mixes are often used to absorb the deformations. Indeed, these loose ma-  
16 terials are well-known for their high-compacting behaviour. An appropriate  
17 modelling of this material must at least be able to reproduce the compaction  
18 behaviour sensitive to the temperature effect.

19 This paper investigates the case of a ramming mix composed of graphite  
20 (80%) and coal tar (20%). Its aspect can be compared with a bituminous  
21 sand. In the literature, many studies were carried out in order to predict  
22 the behaviour of loose geological materials (Bousshine et al., 2001; Liu and  
23 Carter, 2002), pharmaceutical powders (Wu et al., 2005; Han et al., 2008) or  
24 metallic powders (Park et al., 1999; Chtourou et al., 2002). The modelling of  
25 the loose materials whose behaviour is highly dependent on the porosity rate  
26 is classically based on micro-mechanical and macro-mechanical approaches.  
27 The first one assumes that each particle is a sphere. It takes also into account  
28 the contact interactions between the particles (Helle et al., 1985; Fleck et al.,  
29 1992; Biba et al., 1993; Fleck, 1995). From a discrete approach, the macro-  
30 scopic behaviour of a material can be evaluated by means of homogeniza-  
31 tion methods (Piat et al., 2004; Le et al., 2008). Although micro-mechanical  
32 models are devoted to understand the physical behaviour of the constituents,  
33 macro-mechanical models (Shima and Oyane, 1976; Gurson, 1977; Haggblad,  
34 1991) are well-adapted for engineering applications which often occur at a

35 large scale. The phenomenological models were mainly developed for applica-  
36 tions in soil mechanics, such as the Drucker-Prager Cap model (Drucker and  
37 Prager, 1952), the Modified Cam-Clay model (Roscoe and Burland, 1968)  
38 or the Di Maggio-Sandler model (Di Maggio and Sandler, 1971). These Cap  
39 models allow to reproduce the hardening behaviour in compaction of initially  
40 loose powders. In the same approach, many extensions of these models for  
41 porous materials were developed (Chtourou et al., 2002; Aubertin and Li,  
42 2004; Khoei and Azami, 2005; Park, 2007). The Drucker-Prager Cap model  
43 remains one of the most used due to its ability to reproduce shearing and  
44 compaction behaviours. The parameters of this model are simply identi-  
45 fied by carrying classical tests. The hardening behaviour is described by a  
46 Cap parameter dependent on the volumetric inelastic strain (Doremus et al.,  
47 2001; Wu et al., 2005). In these studies, Young's modulus and Poisson's ratio  
48 defining the elastic part are assumed to be constant. In order to reproduce  
49 a possible nonlinear elastic behaviour, some authors have chosen to identify  
50 elastic parameters and yield surface parameters evolving with the relative  
51 density (Kim et al., 2000; Sinka et al., 2003; Michrafy et al., 2004; Han et al.,  
52 2008). The influence of the temperature on the parameters of a Drucker-  
53 Prager Cap model was rarely taken into account (Piret et al., 2004). The  
54 thermal effects were mainly studied in the geotechnical field.

55 Models inspired on the Modified Cam-Clay model are often used to repro-  
56 duce the behaviour of soils such as clays (Liu and Carter, 2002; Piccolroaz  
57 et al., 2006). It is an elastoplastic model with a yield surface which can  
58 grow or shrink according to the hardening rule. Triaxial and die compaction  
59 tests are widely used to identify the shearing and hardening behaviours. The

60 hardening behaviour of the material is written in a specific space in which  
61 only two constant parameters are used to define nonlinear elastic and plastic  
62 parts. Furthermore, this model was extended in order to take into account the  
63 temperature effects (Hueckel and Borsetto, 1990; Hueckel and Baldi, 1990;  
64 Tanaka et al., 1995; Sultan et al., 2002) which mainly modify the size of the  
65 yield surface. In the Modified Cam-Clay model, the size of the yield surface  
66 follows the evolution of the yield stress under hydrostatic compression  $p_c$ .  
67 According to the extensions of the Modified Cam-Clay model, this evolution  
68 can be of exponential or polynomial form with various parameters to identify.  
69 For these reasons, the Modified Cam-Clay model was chosen in this work in  
70 order to reproduce the shearing and hardening behaviours of the ramming  
71 mix sensitive to the temperature effect. No tensile strength, nor cohesion are  
72 introduced in this model due to the context of the study mainly focused on  
73 the compacting behaviour.

74 This paper is organised as follows. Section 2 presents the triaxial and die  
75 compaction tests performed on ramming mix samples with cylindrical shape  
76 allowing the investigation of the shearing and hardening behaviours at dif-  
77 ferent temperatures. A specific calibration step is carried out to know the  
78 radial stress exerted on the sample during the die compaction test. Section  
79 3 describes the adopted Modified Cam-Clay model. Only one parameter is  
80 used to modify the size of the yield surface in order to reproduce the tem-  
81 perature effect. An extension of this model to high-compressive pressures is  
82 also introduced. The various parameters of this model are identified from  
83 the triaxial and die compaction tests data. Finally in section 4, the choice  
84 of the Modified Cam-Clay model is first confirmed by comparison of the nu-

85 merical predictions to experimental data obtained from the tests used for  
86 the identification of the model parameters. Then, a hydrostatic compression  
87 test at room temperature and a die compaction test with heating validate  
88 the ability of this model to reproduce the behaviour of the studied ramming  
89 mix.

## 90 **2. Characterization of the ramming mix behaviour: triaxial and** 91 **die compaction tests**

92 In the high-temperature industry, the ramming mix is used to protect the  
93 steel shell from the expansion of the refractory bricks. Located between the  
94 shell and the bricks, the ramming mix is not directly subjected to the ther-  
95 mal load applied on the masonry. It is mainly stressed in compression at low  
96 strain rates for a temperature range between 20°C and 80°C. Triaxial and die  
97 compaction tests are carried out to identify the shearing and hardening be-  
98 haviours at different temperatures and strain rates. The three-dimensionnal  
99 stress state is defined by two invariants: the equivalent pressure stress  $p$  and  
100 the von Mises equivalent stress  $q$ . They are defined as follows:

$$p = -\frac{1}{3}\text{trace}(\underline{\underline{\sigma}}) \quad (1)$$

101

$$q = \sqrt{\frac{3}{2}\underline{\underline{S}} : \underline{\underline{S}}} \quad (2)$$

102 where  $\underline{\underline{S}}$  is the deviatoric part of the stress tensor  $\underline{\underline{\sigma}}$ , expressed as follows:

$$\underline{\underline{S}} = \underline{\underline{\sigma}} + p\underline{\underline{I}} \quad (3)$$

103  $\underline{\underline{I}}$  is the second order identity tensor.

104 In the case of a cylindrical sample submitted to a compressive axial stress  
105 ( $\sigma_A < 0$ ) and a compressive radial stress ( $\sigma_R < 0$ ), the equivalent pressure  
106 stress reads:

$$p = -\frac{\sigma_A + 2\sigma_R}{3} \quad (4)$$

107 and, the von Mises equivalent stress is:

$$q = |\sigma_A - \sigma_R| \quad (5)$$

### 108 *2.1. Characterization of the shearing behaviour: triaxial tests*

109 Hereafter, we will describe the testing system required to carry out triaxial  
110 tests on the ramming mix. The aim is to characterize the shearing behaviour  
111 of this material. Figure 1 depicts the triaxial compression apparatus used to  
112 perform the triaxial tests. This apparatus is composed of a cell of a 3.5 MPa  
113 capacity, a pressure controller, a 25 mm LVDT sensor and a 10 kN load cell.  
114 The cell is filled with water in order to apply a controlled radial stress on the  
115 sample. This cell is axially free. The axial displacement of the cell which is  
116 measured by the LVDT sensor corresponds to the axial displacement of the  
117 lower part of the sample. The upper part of the sample is motionless which  
118 yields to the axial compression of the sample. The axial force is measured  
119 by the load cell. This testing system ensures that cylindrical samples are  
120 submitted to axial loads under a constant radial pressure imposed by the  
121 water present inside the cell as shown in figure 1.

#### 122 *2.1.1. Triaxial test procedure*

123 The tested samples are 50 mm in diameter and 95 mm in height. Each  
124 specimen is obtained by a manual precompaction of 240 g of carbon ramming

125 mix into a die, up to an average void ratio  $e$  of  $0.42 \pm 0.08$  where  $e$  is given  
126 as follows:

$$e = \frac{\rho_r V}{m} - 1 \quad (6)$$

127  $\rho_r$  is the real density of the material,  $m$  and  $V$  are the mass and the volume  
128 of the sample, respectively. For this material, a gas pycnometer test would  
129 be the most appropriate for the determination of the real density. Here, the  
130 measured real density is  $\rho_r = 1820 \text{ kg/m}^3$ . This value is provided by the Le  
131 Chatelier densimeter test which consists in the immersion of the ramming  
132 mix in oil. Water was avoided to allow a better mixture with the coal tar.  
133 With this apparatus, it is impossible to apply a pressure which could favor  
134 the filling of all the voids by the oil. These voids are located between graphite  
135 grains and coal tar and also in the graphite grains. This grain porosity is the  
136 most complicated to fill by the oil. A mercury porosimetry on the graphite  
137 grains shows there are two kinds of porosities:

- 138 • a grain macroporosity with an average void ratio of  $4 \mu\text{m}$ ,
- 139 • a grain microporosity with an average void ratio of  $0.02 \mu\text{m}$ .

140 A simple relation used in porosimetry tests allow to link the pressure  $P$   
141 exerted on the liquid to fill the voids and the average diameter  $d$  of the  
142 voids:

$$P = \frac{|4\gamma\cos(\alpha)|}{d} \quad (7)$$

143 where  $\gamma$  is the interfacial tension and  $\alpha$  is the wetting angle between the  
144 liquid and the surface of the sample. Considering oil for liquid in the den-  
145 simeter test with  $\gamma=0.03 \text{ N/m}$  and  $\alpha=30^\circ$ , the grain macroporosity is filled

146 with a pressure of 0.025 MPa (lower than the atmospheric pressure) although  
147 5 MPa are required to fill the grain microporosity. So, only the open macro-  
148 porosity in and between grains is filled with the Le Chatelier densimeter test  
149 and so taken into account in the real density measurement. We will consider  
150 a macro void ratio  $e$ .

151 In the case of the triaxial test, the initial macro void ratio of the sample is  
152 heterogeneous due to a manual compaction mode. This sample is set into an  
153 elastomer film to prevent the penetration of water. During the triaxial test,  
154 the sample is first submitted to a constant hydrostatic pressure  $p = -\sigma_R$ .  
155 Then, the axial displacement of the cell at constant velocity and radial pres-  
156 sure implies the increase of the axial stress on the sample following a stress  
157 path defined by the linear relation  $q = 3p$  between the stress invariants.

### 158 *2.1.2. Triaxial test results: temperature and strain rate effects*

159 At room temperature, triaxial tests were carried out for confining pres-  
160 sures from 100 kPa to 800 kPa with velocities of 0.1 mm/min, 0.5 mm/min  
161 and 1 mm/min. To study the influence of the temperature on the shearing  
162 behaviour, a triaxial apparatus operating at high temperatures as used by  
163 Olmo et al. (1996) is necessary. The triaxial apparatus of this study has no  
164 available heating control, therefore a specific procedure was followed. The  
165 sample is first preheated during two hours, at for example 50°C, it is then set  
166 on the triaxial apparatus. In a second step, the cell is filled with preheated  
167 water at 50°C. In order to minimize the decrease of temperature during the  
168 test, a velocity of 1 mm/min was chosen for the triaxial tests at 50°C and  
169 80°C. The strain rate effect is so not considered at elevated temperatures.  
170 For each triaxial test, a critical shearing stress is reached when the increase

171 of the von Mises equivalent stress stops. As presented in figure 2 for triaxial  
172 tests with a confining pressure of 200 kPa, no effect of the strain rate was  
173 observed at room temperature. Only the temperature has an influence on  
174 the critical shearing stress which decreases.

## 175 *2.2. Characterization of the hardening behaviour: die compaction tests*

176 A hydrostatic compression test is classically used to identify the hardening  
177 behaviour of porous materials. An oedometer test on a confined cylindrical  
178 sample constitutes also an acceptable solution but requires to be instru-  
179 mented in order to measure both axial and radial stresses applied on the  
180 sample. The axial stress is easy to get due to a load cell located on the ap-  
181 paratus, whereas the radial stress is applied by the confinement of a hollow  
182 cylinder. In some cases, it is possible to dispose of a load sensor located in  
183 the die in order to have a direct measure (Mesbah et al., 1999; Sinka et al.,  
184 2003). Otherwise, simple relations can be established between the external  
185 circumferential strain of the hollow cylinder and the radial stress applied on  
186 the sample. The identification of this calibration relation is detailed in the  
187 next paragraph.

188 An instrumented die compaction test on a static press INSTRON 5800R was  
189 developed as presented in figure 3. Axial displacement and axial load are  
190 acquired with a 50 mm LVDT sensor and a 250 kN load cell, respectively.  
191 Thanks to a heating control system, compaction tests at elevated tempera-  
192 ture are possible. A heating collar with an integrated K-type thermocouple  
193 linked to the thermal control is set around the free cylinder. A water-cooling  
194 system is added between the load cell and the upper piston in order to avoid  
195 the expansion of the load cell during these tests which could affect the mea-

196 surement. Moreover, a double-effect compaction is chosen (free die in the  
197 axial direction) to compress equally the sample at the top and the bottom.  
198 The die/sample interface is lubricated with a silicon grease to homogenize the  
199 radial stresses applied on the height of the sample. The friction coefficient  
200 between the final compacted sample and the lubricated die was taken to be  
201 0.22 measured with an inclined plane apparatus. This value is an estimation  
202 of the friction coefficient which evolves (decreases globally) when the sample  
203 is compacted due to the decrease of the roughness of the lateral area of the  
204 ramming mix sample. Finally, 8 circumferential gauges are set around the  
205 die in order to evaluate its average circumferential strain. These gauges are  
206 located at mid-height of the die with 70 mm between the lowest and the  
207 highest ones. They allow to record strains of the die for samples higher than  
208 70 mm.

### 209 *2.2.1. Calibration test on a rubbery sample*

210 The calibration relation can be identified experimentally from a test on  
211 an incompressible rubbery sample (Mosbah et al., 1997; Geindreau et al.,  
212 1999; Hong et al., 2008; Michrafy et al., 2009) or numerically when the cir-  
213 cumferential strain of the hollow cylinder is known (Bier et al., 2007). The  
214 calibration test with an incompressible sample is here selected. It is worth  
215 noting that for an incompressible rubbery sample submitted to a confined  
216 load, there is an equality between the axial and radial stresses. In this case,  
217 it is possible to identify a linear relation between the applied hydrostatic  
218 pressure  $P$  and the average of the circumferential strains  $\varepsilon_{\theta\theta}^{die}$  measured by

219 gauges as follows:

$$C(H_{sample}) = \frac{P}{\varepsilon_{\theta\theta}^{die}} \quad (8)$$

220  $C(H_{sample})$  is the sought function which can depend on the height of the  
 221 sample. Figure 4 illustrates the evolution of  $\frac{P}{\varepsilon_{\theta\theta}^{die}}$  as a function of the height  
 222 of the sample. Eight die compaction tests show a constant value for the  $C$   
 223 parameter when the contact between the die and the sample is established.  
 224 These tests carried out on rubbery samples of 102 mm in height (6 tests) and  
 225 92 mm in height (2 tests) allow to identify the following value for  $C$ :

$$C_{test} = 51 \text{ GPa} \quad (9)$$

226 The experimental value can be compared with the theoretical value obtained  
 227 for the case of a hollow cylinder with a height  $L$ , an internal and an external  
 228 radius  $R_1$  and  $R_2$ , respectively. The behaviour of the cylinder is assumed to  
 229 be linear elastic with a Young's modulus  $E$  and a Poisson's ratio  $\nu$ . When  
 230 this cylinder is submitted to an internal pressure  $p_1$ , the local stress field  
 231 (Salençon, 2007) is given by:

$$\sigma_{rr} = \frac{p_1 R_1^2}{R_2^2 - R_1^2} \left( 1 - \left( \frac{R_2}{r} \right)^2 \right) \quad (10)$$

232

$$\sigma_{\theta\theta} = \frac{p_1 R_1^2}{R_2^2 - R_1^2} \left( 1 + \left( \frac{R_2}{r} \right)^2 \right) \quad (11)$$

233 where  $r$  is the local radius ( $R_1 \leq r \leq R_2$ ). Then, taking into account the  
 234 boundary conditions:

$$\sigma_{rr}(r = R_2) = 0 \quad (12)$$

235 and:

$$\sigma_{\theta\theta}(r = R_2) = \frac{2p_1 R_1^2}{R_2^2 - R_1^2} \quad (13)$$

236 Moreover, the constitutive local Hooke's law gives:

$$\varepsilon_{\theta\theta} = \frac{1}{E}\sigma_{\theta\theta} - \frac{\nu}{E}(\sigma_{rr} + \sigma_{zz}) \quad (14)$$

237 It is possible to rewrite equation (14) for  $r = R_2$ :

$$\sigma_{\theta\theta}(r = R_2) = E\varepsilon_{\theta\theta}(r = R_2) + \nu\sigma_{zz}(r = R_2) \quad (15)$$

238 In this theoretical result, the friction between the sample and the cylinder  
239 which induces an axial stress of the cylinder is not taken into account. We  
240 only consider that the upper and lower sections of the steel cylinder are free  
241 which leads to (Salençon, 2007):

$$\sigma_{zz} = 0 \quad (16)$$

242 and so:

$$\sigma_{\theta\theta}(r = R_2) = E\varepsilon_{\theta\theta}(r = R_2) \quad (17)$$

243 Finally, combining equations (13) and (17), we can deduce:

$$C_{theory} = \frac{p_1}{\varepsilon_{\theta\theta}(r = R_2)} = \frac{E}{2} \left( \left( \frac{R_2}{R_1} \right)^2 - 1 \right) \quad (18)$$

244 For the considered die:  $R_1 = 40.6$  mm,  $R_2 = 50$  mm and  $E = 210$  GPa. The  
245 theoretical constant can be evaluated:

$$C_{theory} = 54 \text{ GPa} \quad (19)$$

246 The 6% deviation with the experimental result is due to the friction neglect.  
247 This theoretical value validates the identified experimental one. As a conclu-  
248 sion, this calibration step allows to estimate the average radial stress of the  
249 ramming mix sample during a die compaction test.

250 *2.2.2. Die compaction tests at different temperatures and strain rates*

251 For the compaction tests, 700 g of carbon ramming mix are poured into  
252 a die and precompacted with the apparatus up to an initial macro void  
253 ratio of  $0.37 \pm 0.08$ . The precompaction step induces an axial force close  
254 to 1 kN and a radial stress which allows to maintain the mid-height of the  
255 free cylinder aligned with the mid-height of the initial sample. The initial  
256 sample is 81 mm in diameter and 102 mm in height which gives an aspect ratio  
257 higher than 1 as mentioned in Doremus et al. (2001) in order to minimize the  
258 stress gradients in the sample. This ratio and the double-effect compaction  
259 system allow to assume that the initial macro void ratio is homogeneous.  
260 For tests at elevated temperature, strains are recorded during heating and  
261 dwell time in order to take the expansion into account. Load/unload cycles  
262 are programmed to identify both elastic and plastic parts of the hardening  
263 behaviour. Results of these cycles at different temperatures (20°C, 50°C and  
264 80°C) and velocities (0.1 mm/min and 1 mm/min) are presented in figure 5.  
265 The two curves corresponding to the tests at room temperature at rates  
266 of 0.1 mm/min and 1 mm/min are very close. In the same way, the two  
267 curves at a temperature of 50°C merge. The strain rate has no effect on  
268 the hardening behaviour. The repeatability of the die compaction test is  
269 checked at 80°C and 1 mm/min. Only the temperature seems to have an  
270 influence on the hardening behaviour. For the same consolidation state, the  
271 axial load decreases with the temperature. This phenomenon is observed in  
272 figure 6 where the hardening behaviour with heating is compared with the  
273 one at room temperature. In this test, the sample has an initial macro void  
274 ratio of  $0.34 \pm 0.08$  and the velocity is 0.4 mm/min. During the test, a first

275 heating from 20°C to 50°C and a second one from 50°C to 80°C are imposed  
276 at 3°C/h on the external part of the steel cylinder. Due to the viscous part  
277 of the ramming mix, the increase of temperature induces a slight softening  
278 of the axial stress during a die compaction test.

### 279 *2.2.3. Die compaction tests at high pressures*

280 Figure 7 presents the results of a die compaction test carried out up to  
281 high pressures (40 MPa) at room temperature. The first part of the curve  
282 showing the load/displacement diagram is similar to the plastic part of the  
283 hardening behaviour at room temperature presented in figure 5. In fact,  
284 until a 27 mm axial displacement, the macro void ratio is positive and all  
285 the macro voids in and between the particles are progressively closed. After-  
286 wards due to pressures higher than 4 MPa, the microporosity of the graphite  
287 grains is filled by the coal tar. Considering equation (7) with  $\gamma=0.03$  N/m  
288 and  $\alpha=30^\circ$  which are the parameters for a bituminous, a pressure close to  
289 4 MPa is obtained in order to fill the microporosity of the graphite grains.  
290 The macro void ratio is negative because the real density does not account  
291 for this microporosity.

292

293 As done for the Modified Cam-Clay model, the hardening behaviour is  
294 written in the  $(\ln(p), e)$  plane. In figure 8 are depicted the results of the die  
295 compaction tests carried out at room temperature under low and high pres-  
296 sures. It allows to illustrate how only one elastic slope and two plastic slopes  
297 describe the hardening behaviour of the ramming mix. The formulation of  
298 an extended Modified Cam-Clay model is presented in the following section.

### 299 **3. Modelling of the ramming mix**

#### 300 *3.1. Modified Cam-Clay formulation*

301 For its ability to reproduce the elastic and plastic parts of the harden-  
302 ing behaviour with slopes and the temperature effect, the Modified Cam-  
303 Clay model implemented in the finite element software ABAQUS/Standard  
304 (Abaqus, 2007) is used as the basis of the proposed model. It is an extension  
305 of the original model developed by Roscoe and Burland (1968) introducing  
306 the critical state concept.

##### 307 *3.1.1. Yield surface of the model*

308 The Modified Cam-Clay model described in figure 9 is based on a yield  
309 surface with an associated flow rule:

$$F = \frac{1}{\beta^2} \left( \frac{p}{a} - 1 \right)^2 + \left( \frac{q}{Ma} \right)^2 - 1 = 0 \quad (20)$$

310 where  $M$  is a constant that defines the slope of the critical state line,  $a$  is the  
311 size of the yield surface and  $\beta$  is a constant which can modify the shape of  
312 the cap. This parameter is here used to take into account the temperature  
313 effect on the yield surface of the hardening behaviour. It is worth noting that  
314 for a hydrostatic compression  $p_c$ , the yield stress is given by:

$$p_c(T) = a(1 + \beta(T)) \quad (21)$$

315 The yield surface (equation (20)) is written in terms of two stress invariants:  
316 the equivalent pressure stress  $p$  and the von Mises equivalent stress  $q$ . In this  
317 section, we introduce some notations. The strain tensor  $\underline{\underline{\varepsilon}}$  is decomposed in  
318 two parts as follows:

$$\underline{\underline{\varepsilon}} = \underline{\underline{\varepsilon}}^{el} + \underline{\underline{\varepsilon}}^{pl} \quad (22)$$

319  $\underline{\underline{\varepsilon}}^{el}$  denotes the elastic part of the strain and  $\underline{\underline{\varepsilon}}^{pl}$  represents its plastic part.  
 320 Each of these strains can be divided into deviatoric and volumetric parts.  
 321 The strain field (equation (22)) reads then:

$$\underline{\underline{\varepsilon}} = (\underline{\underline{\varepsilon}}_{dev}^{el} + \underline{\underline{\varepsilon}}_{dev}^{pl}) - (\varepsilon_{vol}^{el} + \varepsilon_{vol}^{pl})\underline{\underline{I}} \quad (23)$$

322 Moreover, the volumetric part of the strain field  $\varepsilon_{vol}$  can be expressed as a  
 323 function of the macro void ratio  $e$  and the initial macro void ratio  $e_0$ :

$$\varepsilon_{vol} = \varepsilon_{vol}^{el} + \varepsilon_{vol}^{pl} = \ln \left( \frac{1+e}{1+e_0} \right) \quad (24)$$

324 The strain field will evolve following a hardening behaviour for high  $p$  values  
 325 and a shearing behaviour for high  $q$  values.

### 326 3.1.2. Shearing behaviour

327 The critical state line is the key point of the model because it allows to  
 328 define hardening and softening behaviours of the material. In fact, when  
 329 submitted to an increasing deviatoric stress for a constant value of  $p$  lower  
 330 than  $a$ , the material exhibits a deviatoric elastic behaviour defined by:

$$d\underline{\underline{S}} = 2Gd\underline{\underline{\varepsilon}}_{dev}^{el} \quad (25)$$

331 where  $G$  is the shear modulus whose expression is defined by:

$$G = \frac{3(1-2\nu)(1+e_0)}{2(1+\nu)\kappa} \exp(\varepsilon_{vol}^{el})p \quad (26)$$

332  $\kappa$  is the logarithmic bulk modulus. Equation (26) demonstrates that  $G$  in-  
 333 creases as the material is compacted. Then, the yield surface is reached for  
 334 values of  $q$  upper than the product  $Mp$  leading to a softening behaviour. The  
 335 yield stress curve decreases and reaches the critical state line.

336 *3.1.3. Hardening behaviour*

337 When submitted to hydrostatic pressures, the elastic part is governed by  
 338 the volumetric behaviour of the material as follows:

$$\exp(\varepsilon_{vol}^{el}) = 1 + \frac{\kappa}{1 + e_0} \ln \frac{p_0}{p} \quad (27)$$

339 where  $p_0$  is the initial value of the equivalent pressure stress. If yielding  
 340 occurs, the material exhibits a hardening behaviour described in figure 10  
 341 and defined as:

$$de = -\lambda d(\ln p) \quad (28)$$

342  $\lambda$  is the logarithmic hardening constant for the plasticity. Hereafter, this  
 343 slope takes the value of  $\lambda_1$  for positive macro void ratios and  $\lambda_2$  for negative  
 344 macro void ratios. The hardening behaviour was implemented thanks to  
 345 a user defined field (USDFLD) subroutine which computes the macro void  
 346 ratio of each element constituting the mesh. Then, two specific logarithmic  
 347 hardening constants function of the macro void ratio are defined.

348 The hardening behaviour is characterized by the growth of the yield surface  
 349 size defined by:

$$a = a_0 \exp \left[ (1 + e_0) \frac{1 - \exp(\varepsilon_{vol}^{pl})}{\lambda - \kappa \exp(\varepsilon_{vol}^{pl})} \right] \quad (29)$$

350 where  $a_0$  is the initial position of  $a$ . This initial overconsolidation is computed  
 351 as follows:

$$a_0 = \frac{1}{2} \exp \left( \frac{e_1 - e_0 - \kappa \ln p_0}{\lambda - \kappa} \right) \quad (30)$$

352  $e_1$  denotes the macro void ratio of the ramming mix for a hydrostatic pressure  
 353 of 1 MPa. If the critical state line is reached after the yielding, the material  
 354 then distorts without any changes in shear stress or volume.

355 Finally, the use of the Modified Cam-Clay model requires 9 parameters:

- 356 •  $\nu$  and  $\kappa$  for the elastic behaviour,
- 357 •  $\lambda_1$  and  $\lambda_2$  for the plastic behaviour,
- 358 •  $M$  for the critical state line,
- 359 •  $\beta$  for the yield surface evolving with the temperature,
- 360 •  $e_0$ ,  $e_1$  and  $p_0$  to define the initial state of the material.

### 361 *3.2. Parameters identification*

362 For different temperatures, the shearing parameter  $M$  is identified from  
363 the triaxial tests carried out and described in section 2.1.2. Results of the  
364 die compaction tests (section 2.2.2) written in the  $(\ln(p), e)$  plane allow to  
365 obtain the parameters of the hardening behaviour. The temperature effect  
366 on the parameters of this model is particularly observed.

#### 367 *3.2.1. The critical state line*

368 From the results of the triaxial tests mentioned in section 2.1.2, the  
369 critical shearing stresses permit to plot the critical state line. Considering no  
370 effect of the velocity, a single value of  $M$  was identified at room temperature:  
371  $M_{20^\circ C} = 1.18$  as presented in figure 11. Results of tests carried out at  $50^\circ C$   
372 and  $80^\circ C$  are depicted in the same figure. All of them are located under the  
373 critical state line identified at room temperature. Since there is no thermal  
374 control during these tests, we can only conclude the global influence of the  
375 temperature on the slope of the critical state line but nothing on the real  
376 value of the parameter  $M$ . For this reason, a slight decrease of the slope was  
377 assumed for this range of temperatures:  $M_{50-80^\circ C} = 1.1$ .

378 *3.2.2. Parameters of the hardening behaviour*

379 Parameters identification is done in the  $(\ln(p), e)$  plane as previously ex-  
380 plained. Results of the carried die compaction tests at different temperatures  
381 are exploited as in figure 8. The parameters of the hardening behaviour are  
382 reported in table 1. Temperature has no effect on the elastic and plastic  
383 slopes. As shown in figure 12, only the parameter  $e_1$  decreases with the in-  
384 crease of the temperature. It means that for the same hydrostatic pressure  
385 (1 MPa), the sample is more compacted at 80°C than at 20°C. This result is  
386 directly related to a phenomenon often observed (Hueckel and Baldi, 1990;  
387 Sultan et al., 2002): the decrease of the yield surface with respect to the  
388 temperature. As written in equation (30),  $e_1$  is only used to compute the ini-  
389 tial consolidation state at the corresponding temperature. In this paper, the  
390 temperature effect is reproduced with the parameter  $\beta$  which can modify the  
391 shape of the cap part. A unit value is set at room temperature:  $\beta(20^\circ C) = 1$ .  
392 In order to assess  $\beta(50^\circ C)$  and  $\beta(80^\circ C)$ , we consider a hydrostatically com-  
393 pacted sample heated from the room temperature up to a temperature  $T$   
394 without any volume change. It means that during this heating step the con-  
395 solidation state  $a$  and the void ratio  $e$  remain constant. From equation (21),  
396 we can write:

$$\beta(T) = \frac{p_c(T)}{a(T)} - 1 \quad (31)$$

397 At room temperature:

$$\beta(20^\circ C) = \frac{p_c(20^\circ C)}{a(20^\circ C)} - 1 = 1 \quad (32)$$

398 Considering the constant consolidation state, it comes at higher tempera-  
399 tures:

$$\beta(T) = \frac{p_c(T)}{a(20^\circ C)} - 1 = 2 \frac{p_c(T)}{p_c(20^\circ C)} - 1 \quad (33)$$

400 Equation (28) and the knowledge of parameters  $e_1$ ,  $\lambda_1$ ,  $\lambda_2$  reported in table 1  
401 allow to compute the values  $p_c(50^\circ C)$  and  $p_c(80^\circ C)$  from a chosen value of  
402  $p_c(20^\circ C)$  after heating without any volume change. The computation of  $\beta$   
403 (equation (33)) provides the following values at  $50^\circ C$  and  $80^\circ C$ :

$$\beta(50^\circ C) = 0.56 \quad \text{and} \quad \beta(80^\circ C) = 0.21 \quad (34)$$

404 All the parameters needed for the computation of the Modified Cam-Clay  
405 model for the ramming mix are presented in table 2. The Poisson's ratio  $\nu$  is  
406 set equal to a classical value of 0.25. The obtained results are in agreement  
407 with the tendencies of the evolution of the Cam-Clay parameters with tem-  
408 perature as mentionned in Graham et al. (2001). In fact, heating produces  
409 variations of the yield surface size leading to a decrease of the elastic zone.

#### 410 4. Numerical results

411 Axisymmetric numerical analysis of triaxial and die compaction tests  
412 used for parameters identification is carried out with the finite element code  
413 ABAQUS/Standard 6.7. Two additional tests independent from the previous  
414 ones are also modelled in order to validate the model. For all these simula-  
415 tions, the ramming mix behaviour is defined with the parameters previously  
416 identified.

417 *4.1. Modelling of identification tests*

418 To simulate triaxial and compaction tests, the ramming mix sample is  
419 meshed with four-node axisymmetric continuum elements. Upper and lower  
420 pistons are modelled with rigid supports.

421 *4.1.1. Triaxial tests*

422 For the triaxial test, a hard-contact condition in normal behaviour and a  
423 friction coefficient of 0.3 (measured with an inclined plane apparatus between  
424 the filter paper and the ramming mix) in tangential behaviour is defined  
425 between the sample and pistons. Due to the manual compaction mode, an  
426 initial heterogeneous macro void ratio is considered for the sample with an  
427 average value of 0.42. The heterogeneous distribution of the macro void  
428 ratio is assumed to be linear with  $e = 0.52$  at the bottom of the sample  
429 and  $e = 0.32$  at its top. In fact, the initial sample is more compacted in its  
430 upper part in order to respect the chosen initial height. This distribution was  
431 chosen due to its efficiency to accurately reproduce the shearing behaviour  
432 under a radial stress of 100 kPa. As shown in figure 13(a), the sample is  
433 first hydrostatically stressed on its upper and lateral areas. Then, an axial  
434 displacement is imposed on the lower piston. The axial load is read on the  
435 upper piston which is locked. This load corresponds to the one measured by  
436 the load cell in figure 1. In fact for the simulation, the axial pressure exerted  
437 by water is directly applied on the sample and not on the upper piston. When  
438 compacted, the sample takes a convex shape (see figure 13(b)) and is mainly  
439 stressed in the center and the corners. Numerical and experimental results  
440 compare the evolution of the axial load with respect to the axial displacement.  
441 They are shown in figure 14 at room temperature and in figure 15 at higher

442 temperatures (50°C and 80°C). A good agreement is reached between tests  
443 and simulations for the shearing behaviour.

#### 444 *4.1.2. Die compaction tests*

445 In the die compaction test modelling presented in figure 16(a), the steel  
446 die is meshed with four-node axisymmetric continuum elements. Its be-  
447 haviour is assumed to be isotropic elastic linear with a Young's modulus  
448  $E = 210$  GPa and a Poisson's ratio  $\nu = 0.3$ . Contact conditions are de-  
449 fined between the steel parts and the sample: a hard-contact condition in  
450 normal behaviour and a friction coefficient of 0.22 in tangential behaviour as  
451 explained in section 2.2. Due to the double-effect precompaction step, a ho-  
452 mogeneous macro void ratio of 0.37 describes the initial state of the sample.  
453 It is submitted to an axial displacement imposed by the lower piston. The  
454 upper piston is locked and provides the axial load applied on the sample. As  
455 shown in figure 16(b), the radial stress distribution in the sample is globally  
456 homogeneous. Moreover, the steel die which axially moves up due to the  
457 friction, is radially stressed in compression where it is in contact with the  
458 sample and in tensile elsewhere. The axial load response to the axial imposed  
459 displacement is studied. As shown in figure 17, elastic and plastic parts of  
460 the hardening behaviour accurately fit with the proposed model at room  
461 temperature. The same agreement between experimental and numerical re-  
462 sults is obtained as well at 50°C and 80°C. Figure 18 shows the necessity to  
463 allow for a second plastic slope  $\lambda_2$  because it permits to define the hardening  
464 behaviour at high pressures. In order to avoid numerical problems, a sweet  
465 transition is defined between the two plastic slopes.

466 *4.2. Validation of the model*

467 *4.2.1. Die compaction test with loads/unloads and heating*

468 The first test used for the validation is a die compaction test with load/unload  
469 cycles and heating during the test. A sample with an initial homogeneous  
470 macro void ratio of 0.37 is submitted to a load/unload cycle at a velocity of  
471 0.2 mm/min and an increase of the temperature in two steps: first from 20°C  
472 to 50°C and then from 50°C to 80°C as shown in figure 19. This thermal  
473 field was measured by the thermocouple during the test and applied in the  
474 simulation on the external part of the free cylinder. Thanks to a preliminary  
475 thermal computation in transient state with the material properties defined  
476 in table 3, the evolution of the temperature in the ramming mix sample is  
477 known. Numerical and experimental results for this test are depicted in fig-  
478 ure 20. The efficiency of the model to reproduce accurately the hardening  
479 behaviour of the material evolving with the temperature is validated.

480 *4.2.2. Hydrostatic compression test*

481 A second test using the triaxial apparatus is developed as well. The  
482 idea is to carry out a hydrostatic compression test with loads and unloads,  
483 with the only use of the pressure exerted by water. For the ramming mix  
484 sample preparation, a similar procedure to that described for triaxial tests  
485 was adopted. As shown in figure 21, the cell is axially locked like the upper  
486 piston which has no contact with the sample. So no axial displacement is  
487 applied on the sample and no axial load is measured by the load cell. The  
488 sample is put on the lower locked piston and a cylindrical part is set above  
489 in order to attach the elastomer film. The sample is then hydrostatically  
490 stressed on the lateral and upper surfaces. In order to evaluate the way

491 the sample distorts, the axial displacement of the center of the cylindrical  
492 part above the sample is investigated. The sample being in the cell filled  
493 with water, a direct measurement is impossible. For this reason a marker  
494 tracking method (Nugent et al., 2000; Lee et al., 2009) is chosen. Markers are  
495 drawn on the cylindrical part. A 8-bit CCD camera allows to grab images  
496 of the marked area for each hydrostatic stress increment of 50 kPa. The  
497 displacement field of the center of the cylindrical part can be computed. We  
498 assume that this displacement corresponds to the one of the upper part of  
499 the sample. Figure 22 shows the experimental and numerical evolution of  
500 the axial displacement of the upper part of the sample with the hydrostatic  
501 stress. The plastic part is well-reproduced although the elastic part observed  
502 is not accurately numerically obtained. It is due to the fact that during the  
503 unload, the cylindrical part goes up and loses the contact with the upper  
504 part of the sample. It means that the axial displacement of the cylindrical  
505 part is different from the axial displacement of the upper part of the sample  
506 at the end of the unload. Moreover during the unload, a pump-effect leads  
507 to the filling of the space between the cylindrical part and the upper part of  
508 the sample with air. So during the reload, the air is first evacuated and then  
509 the sample loaded. This phenomenon is not numerically taken into account  
510 which leads to slight differences for the elastic part.

## 511 **5. Conclusion**

512 In the present paper, a specific ramming mix of the steel industry was  
513 studied in order to select an appropriate model reproducing its behaviour.  
514 A macroscopic model which is well adapted to engineering applications was

515 retained. It is the Modified Cam-Clay model which is often used in the  
516 geotechnical field due to its ability to reproduce the shearing and hardening  
517 behaviours. In the proposed model, a single parameter  $\beta$  permits to take  
518 into account the temperature effect on the hardening behaviour. Moreover,  
519 the hardening behaviour was completed for high pressures defining a second  
520 plastic slope. This second slope is related to the microporosity filling. An  
521 experimental campaign was lead to determine the material parameters of the  
522 proposed model. Triaxial tests allowed to identify the slope of the critical  
523 state line of the model and an instrumented die compaction test was con-  
524 ceived to characterize the hardening behaviour. For all these tests, no influ-  
525 ence of the strain rate was established for strain rates lower than  $2 \times 10^{-4} \text{ s}^{-1}$ .  
526 Only the temperature effect on the critical state line and the yield surface  
527 was clearly observed. In fact, the size of the yield surface decreases while  
528 temperature increases. In order to validate the agreement reached between  
529 experimental and numerical results, two additional tests were carried out.  
530 A die compaction test with heating and a hydrostatic compression test at  
531 room temperature coupled with a marker tracking method have confirmed  
532 the relevance of the model to reproduce the behaviour of the ramming mix.

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659 **List of Figures**

660	1	Triaxial compression test. . . . .	34
661	2	Triaxial tests with $ \sigma_R  = 200$ kPa at different temperatures	
662		and velocities. . . . .	35
663	3	Instrumented die compaction test. . . . .	36
664	4	Identification of the calibration relation. . . . .	37
665	5	Results of die compaction tests at different velocities and tem-	
666		peratures. . . . .	38
667	6	Temperature effect on the hardening behaviour. . . . .	39
668	7	Die compaction test under high pressures. . . . .	40
669	8	Hardening behaviour in the $(p, q)$ plane. . . . .	41
670	9	Yield surface and critical state line of the Modified Cam-Clay	
671		model. . . . .	42
672	10	Compaction behaviour of the Modified Cam-Clay model. . . . .	43
673	11	Results of triaxial tests at 3 different temperatures. . . . .	44
674	12	Influence of the temperature on the hardening behaviour. . . . .	45
675	13	Simulation of the triaxial test: (a) Mesh and boundary condi-	
676		tions, (b) Axial stress distribution. . . . .	46
677	14	Triaxial tests with different radial stresses at $T=20^\circ\text{C}$ . . . . .	47
678	15	Triaxial tests at $T=50^\circ\text{C}$ and $T=80^\circ\text{C}$ . . . . .	48
679	16	Simulation of the die compaction test: (a) Mesh and boundary	
680		conditions, (b) Radial stress distribution. . . . .	49
681	17	Die compaction test at $T=20^\circ\text{C}$ . . . . .	50
682	18	Experimental and numerical results for high pressures. . . . .	51
683	19	Temperature evolution during the die compaction test. . . . .	52

684	20	Die compaction test with loads/unloads and heating. . . . .	53
685	21	Hydrostatic compression test. . . . .	54
686	22	Hydrostatic compression test with loads/unloads. . . . .	55

687 **List of Tables**

688	1	Hardening behaviour parameters. . . . .	56
689	2	Modified Cam-Clay model parameters. . . . .	57
690	3	Thermal properties. . . . .	58

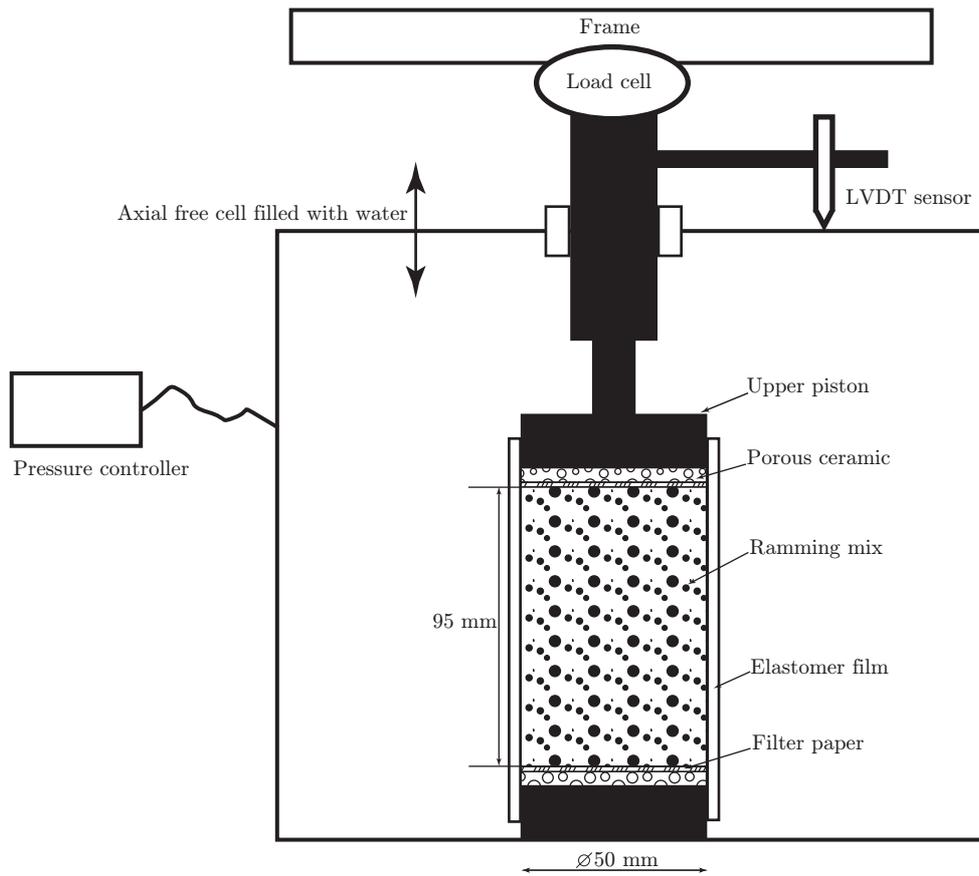


Figure 1: Triaxial compression test.

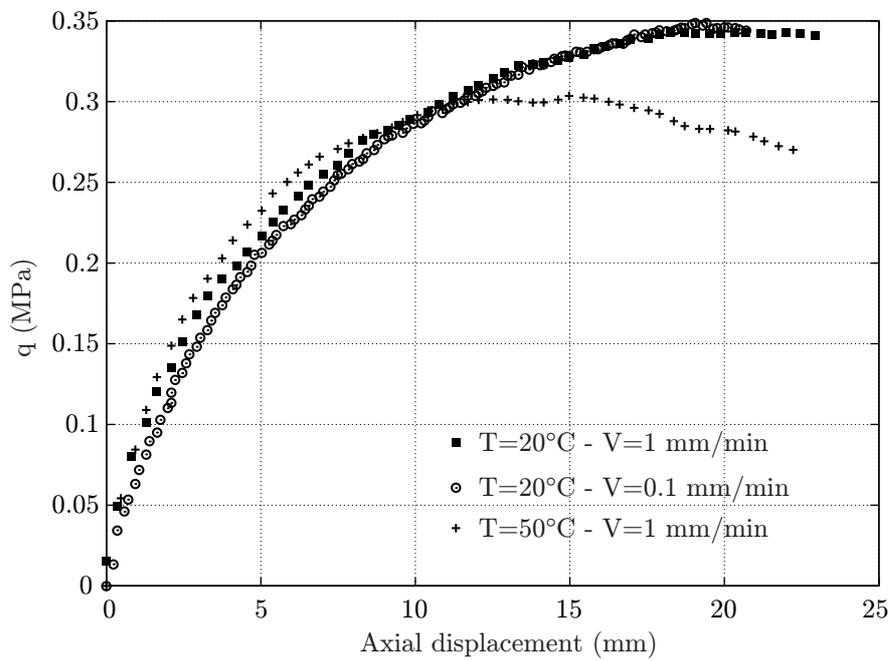


Figure 2: Triaxial tests with  $|\sigma_R| = 200$  kPa at different temperatures and velocities.

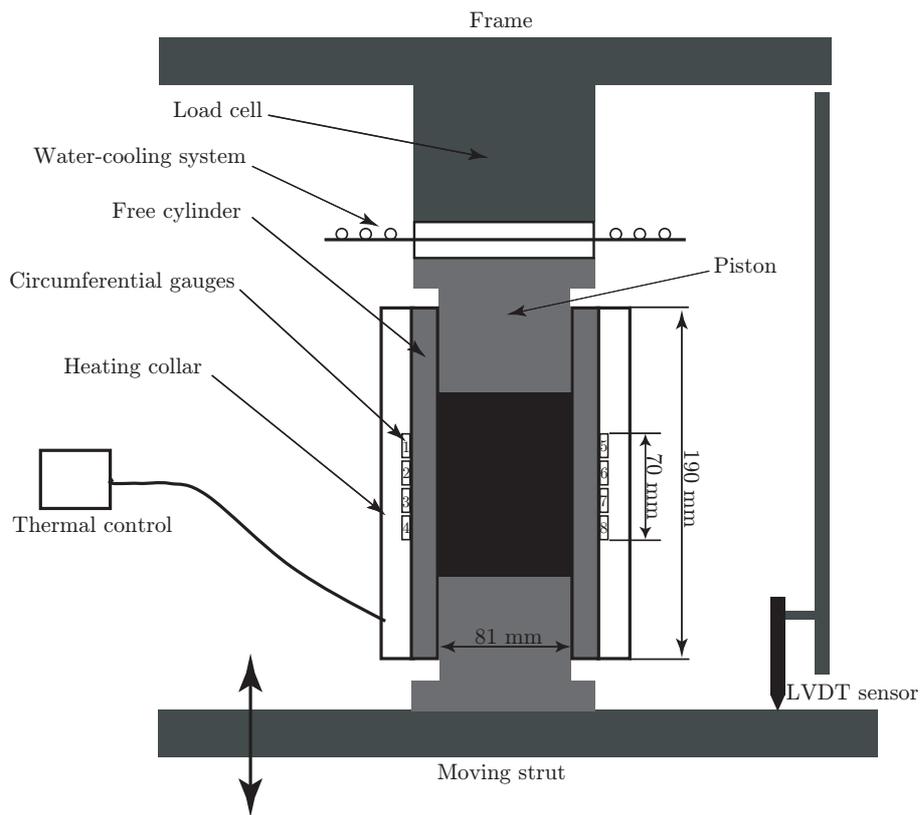


Figure 3: Instrumented die compaction test.

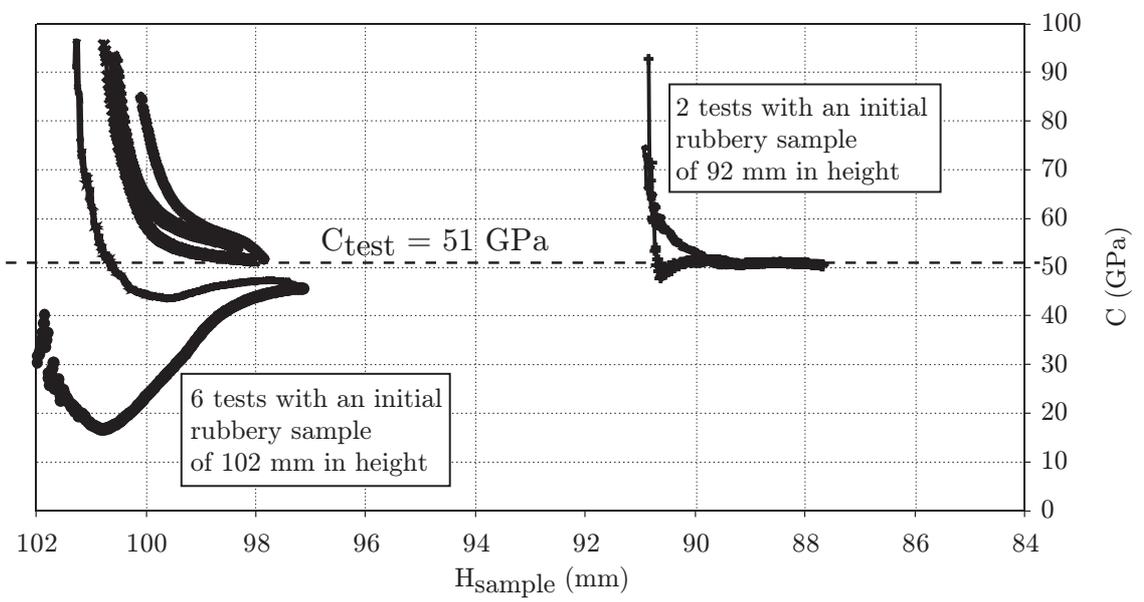


Figure 4: Identification of the calibration relation.

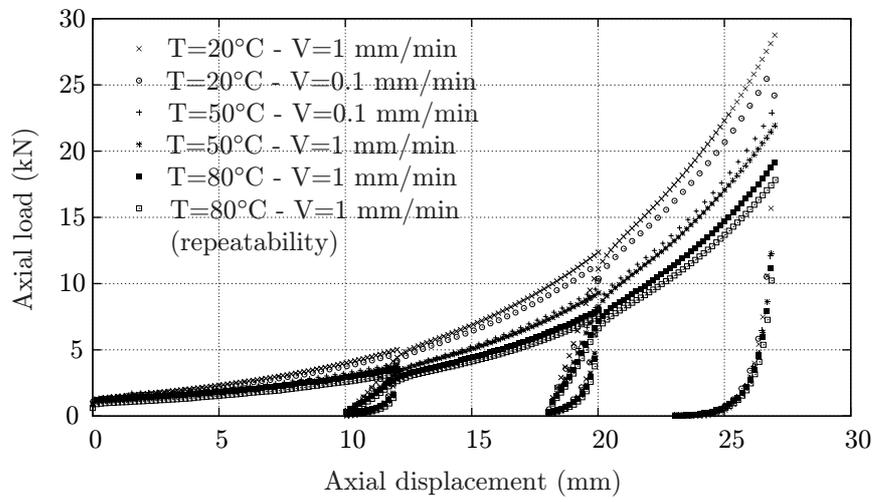


Figure 5: Results of die compaction tests at different velocities and temperatures.

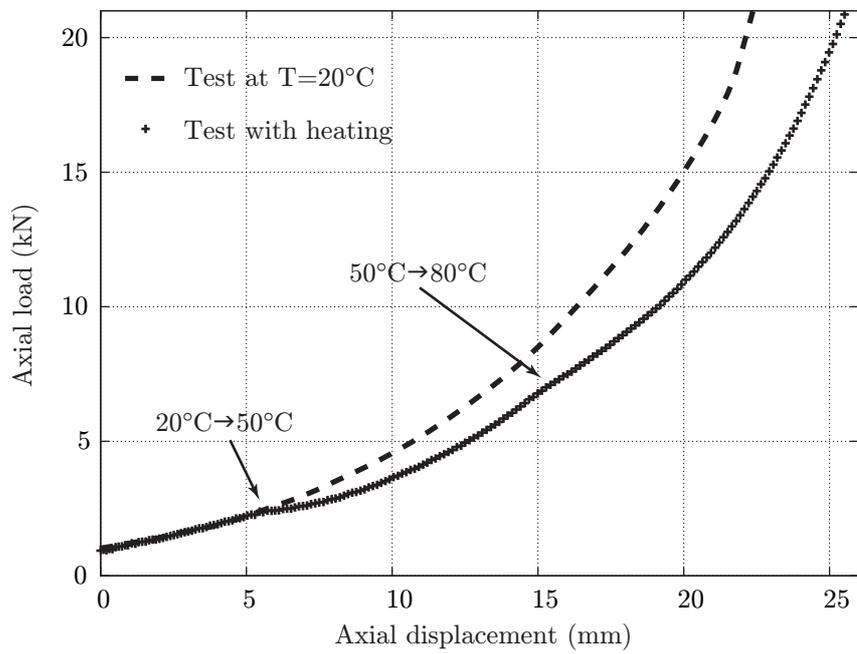


Figure 6: Temperature effect on the hardening behaviour.

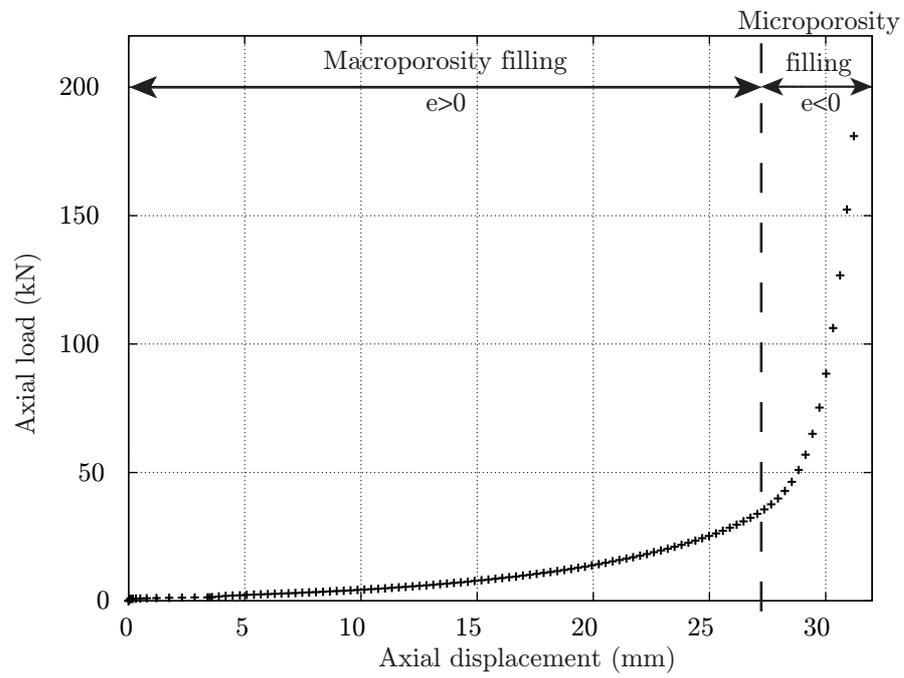


Figure 7: Die compaction test under high pressures.

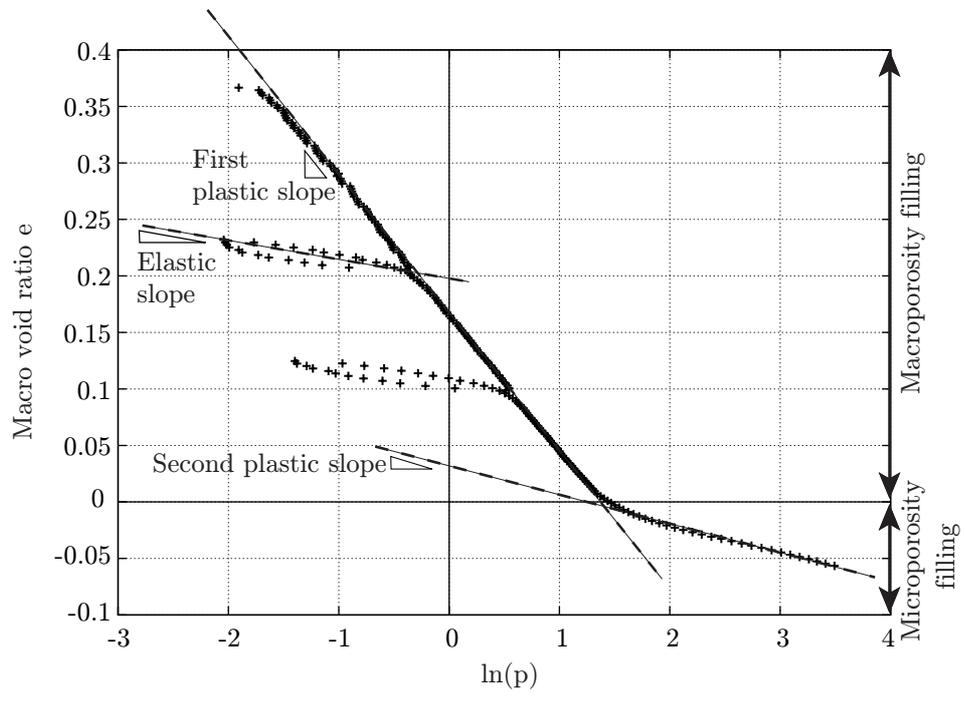


Figure 8: Hardening behaviour in the  $(p, q)$  plane.

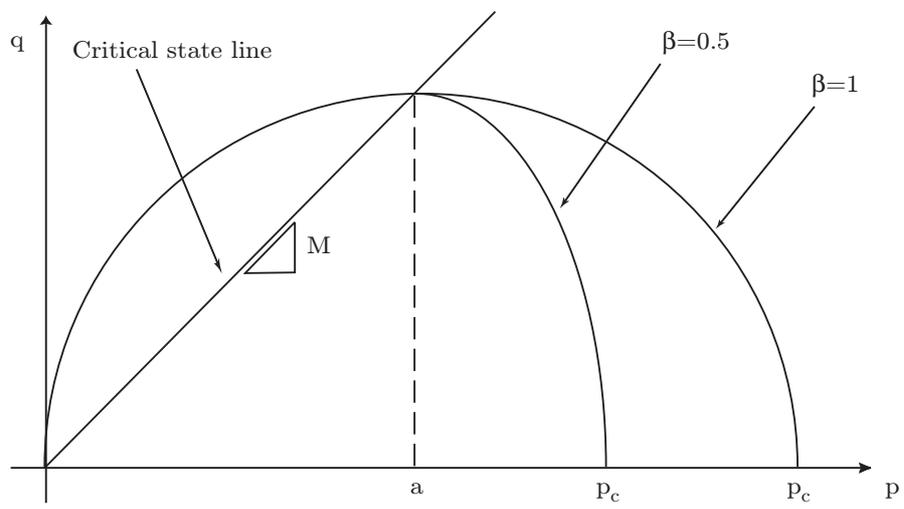


Figure 9: Yield surface and critical state line of the Modified Cam-Clay model.

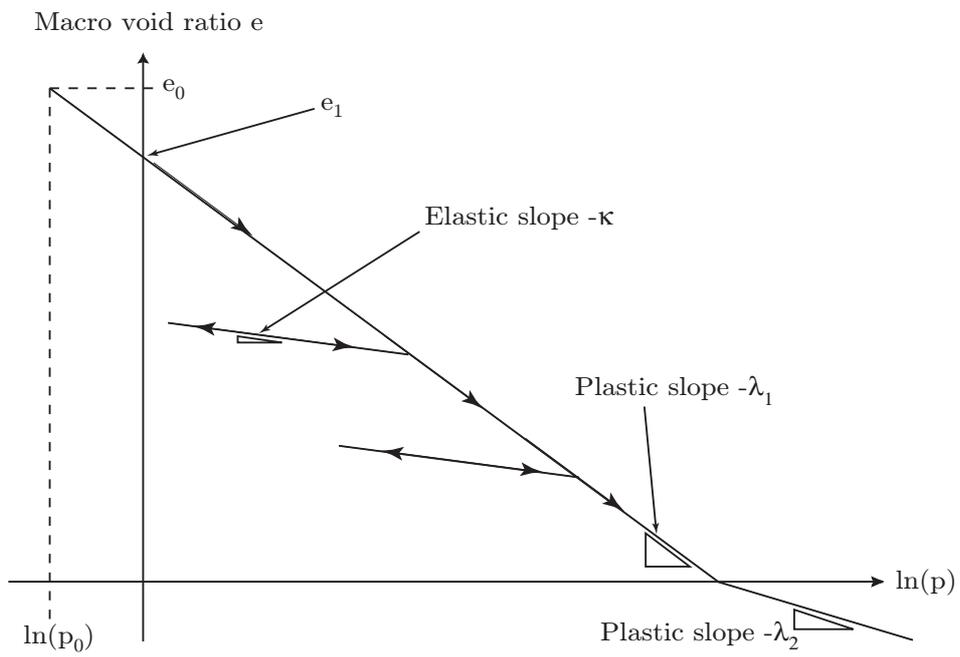


Figure 10: Compaction behaviour of the Modified Cam-Clay model.

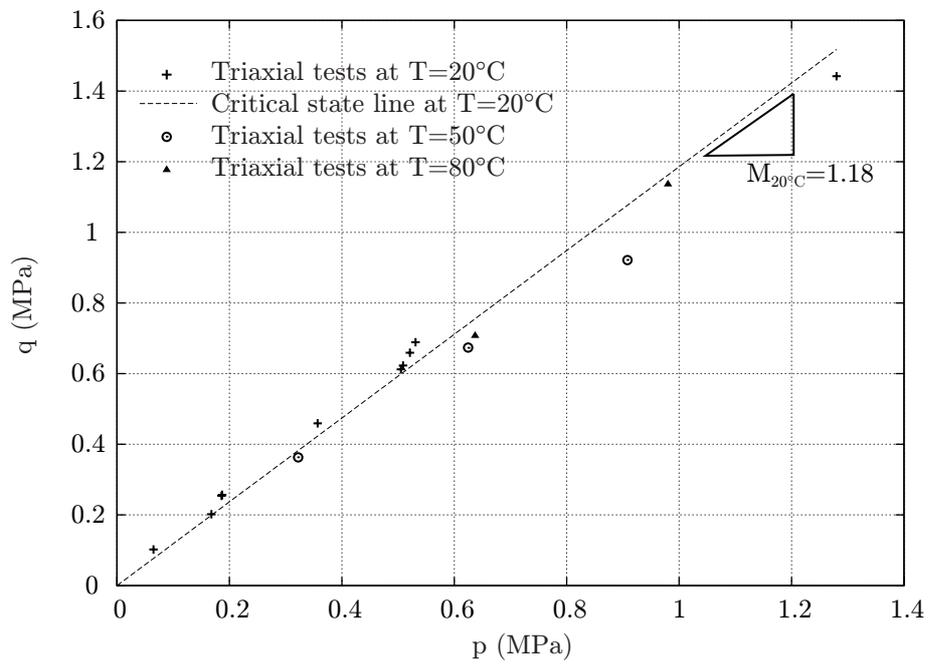


Figure 11: Results of triaxial tests at 3 different temperatures.

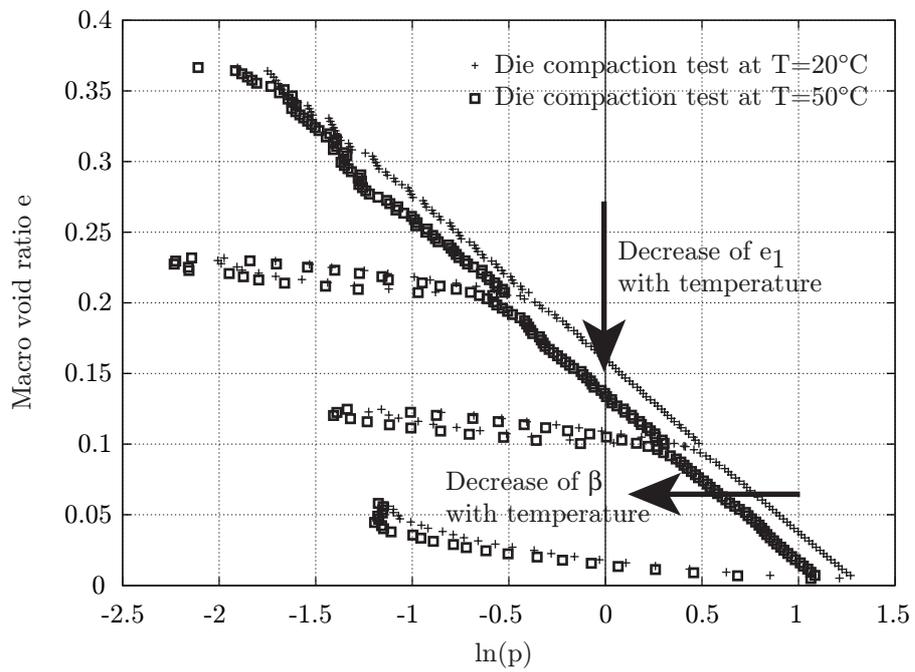


Figure 12: Influence of the temperature on the hardening behaviour.

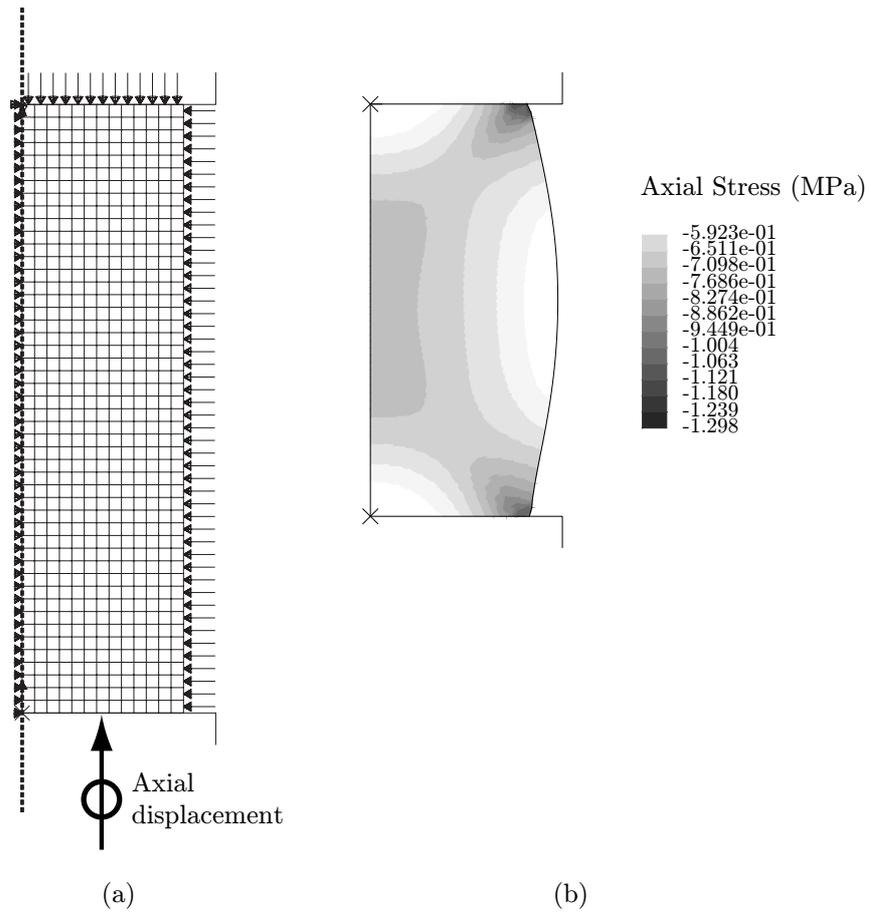


Figure 13: Simulation of the triaxial test: (a) Mesh and boundary conditions, (b) Axial stress distribution.

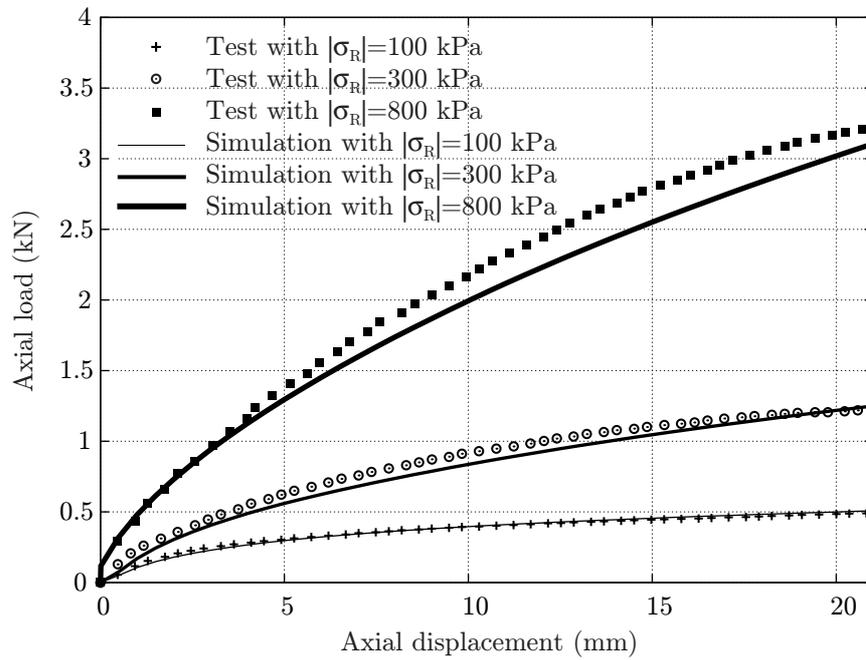


Figure 14: Triaxial tests with different radial stresses at  $T=20^\circ\text{C}$ .

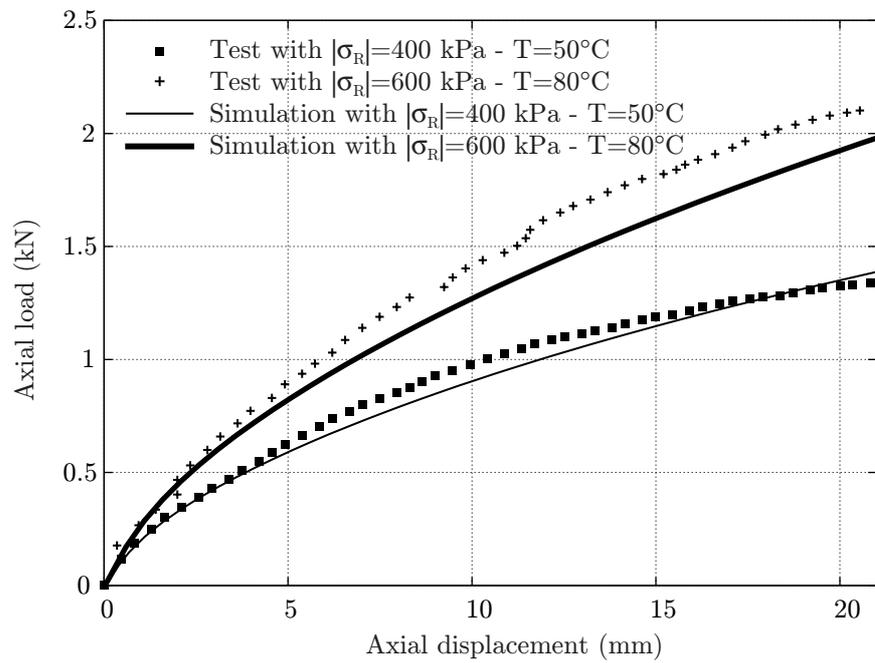


Figure 15: Triaxial tests at  $T=50^\circ\text{C}$  and  $T=80^\circ\text{C}$ .

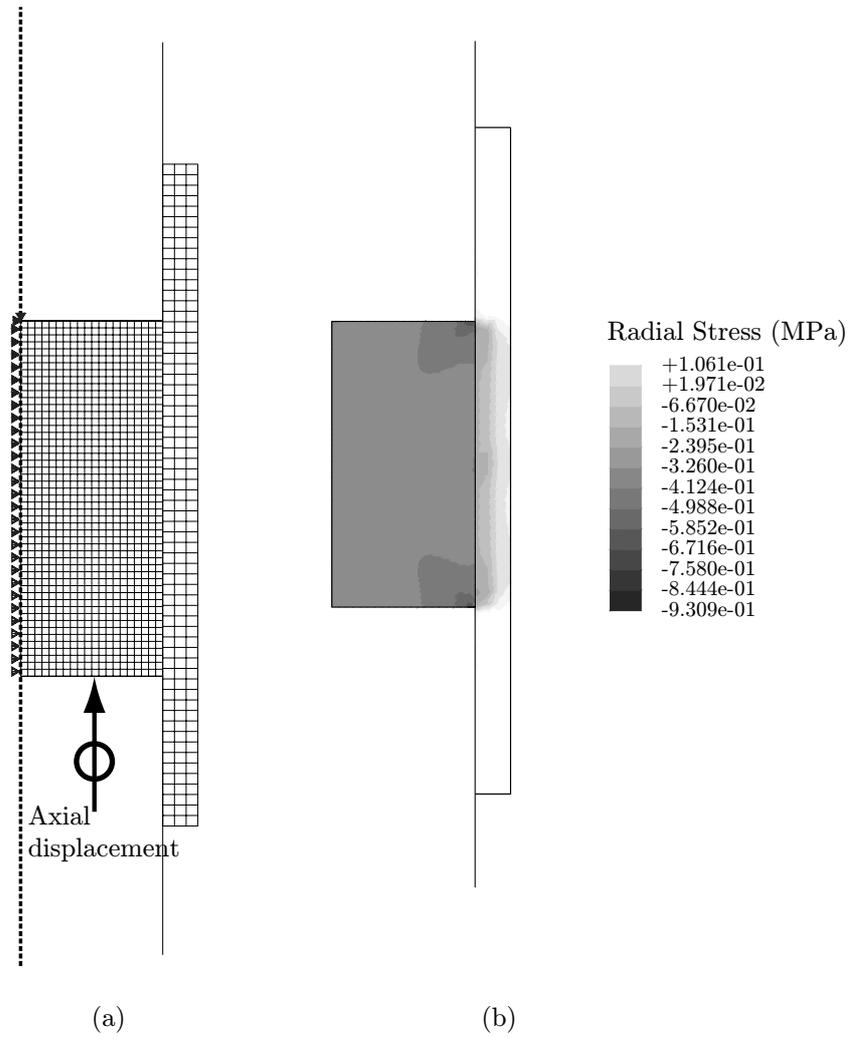


Figure 16: Simulation of the die compaction test: (a) Mesh and boundary conditions, (b) Radial stress distribution.

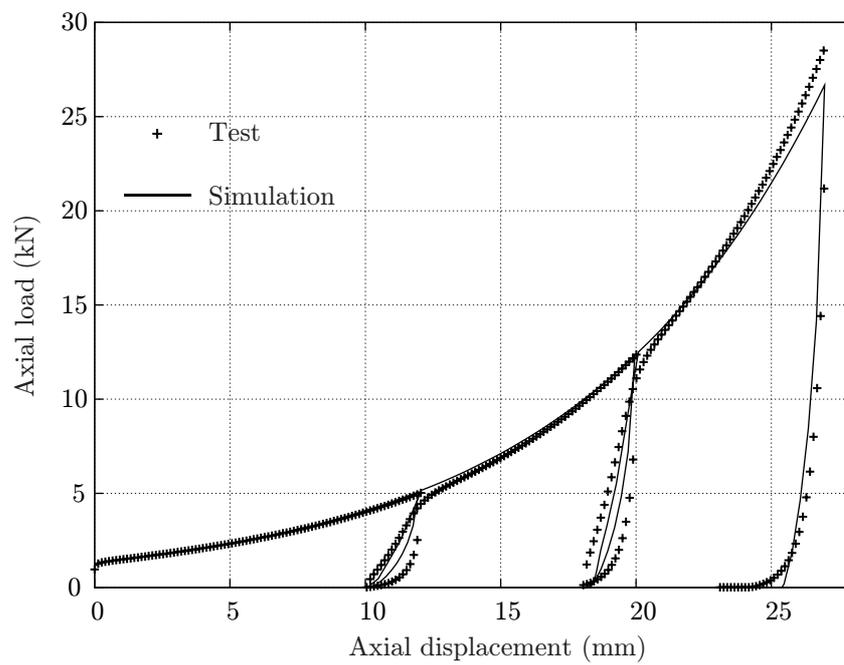


Figure 17: Die compaction test at  $T=20^{\circ}\text{C}$ .

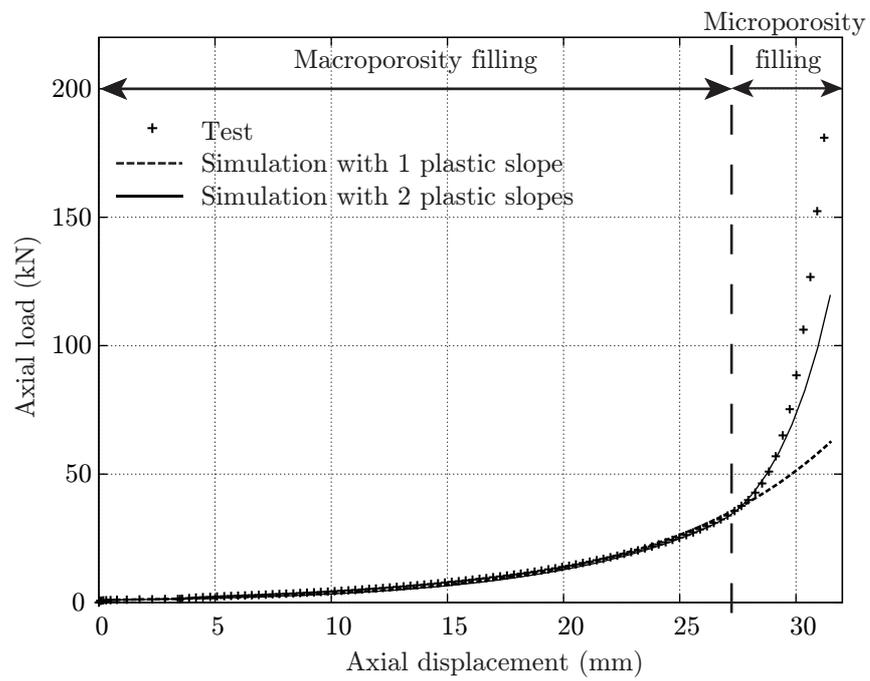


Figure 18: Experimental and numerical results for high pressures.

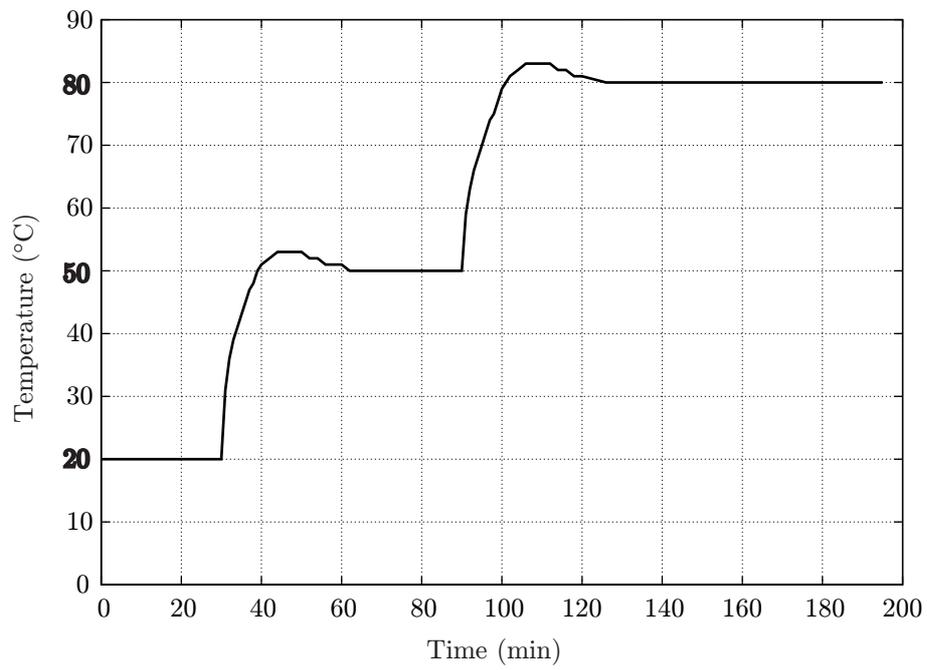


Figure 19: Temperature evolution during the die compaction test.

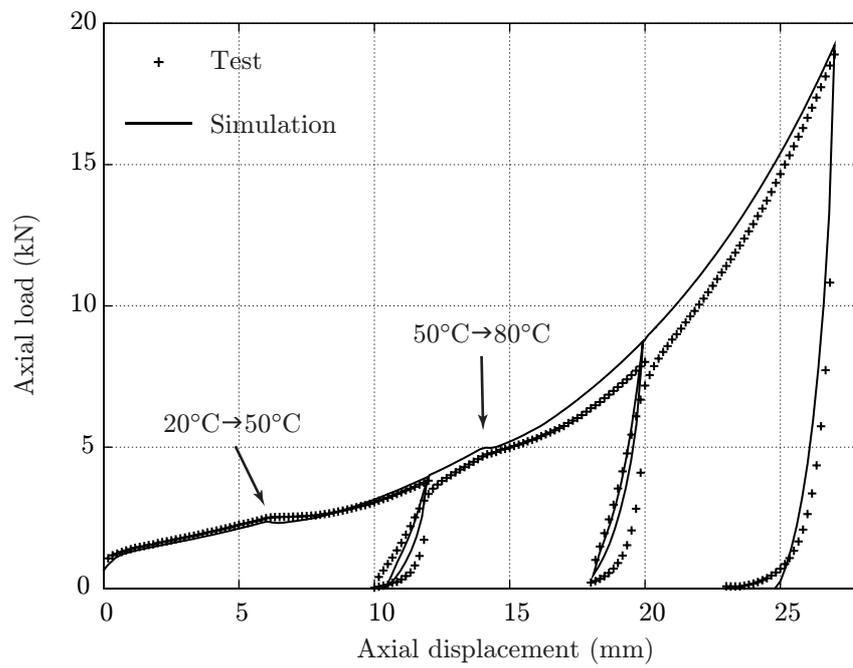


Figure 20: Die compaction test with loads/unloads and heating.

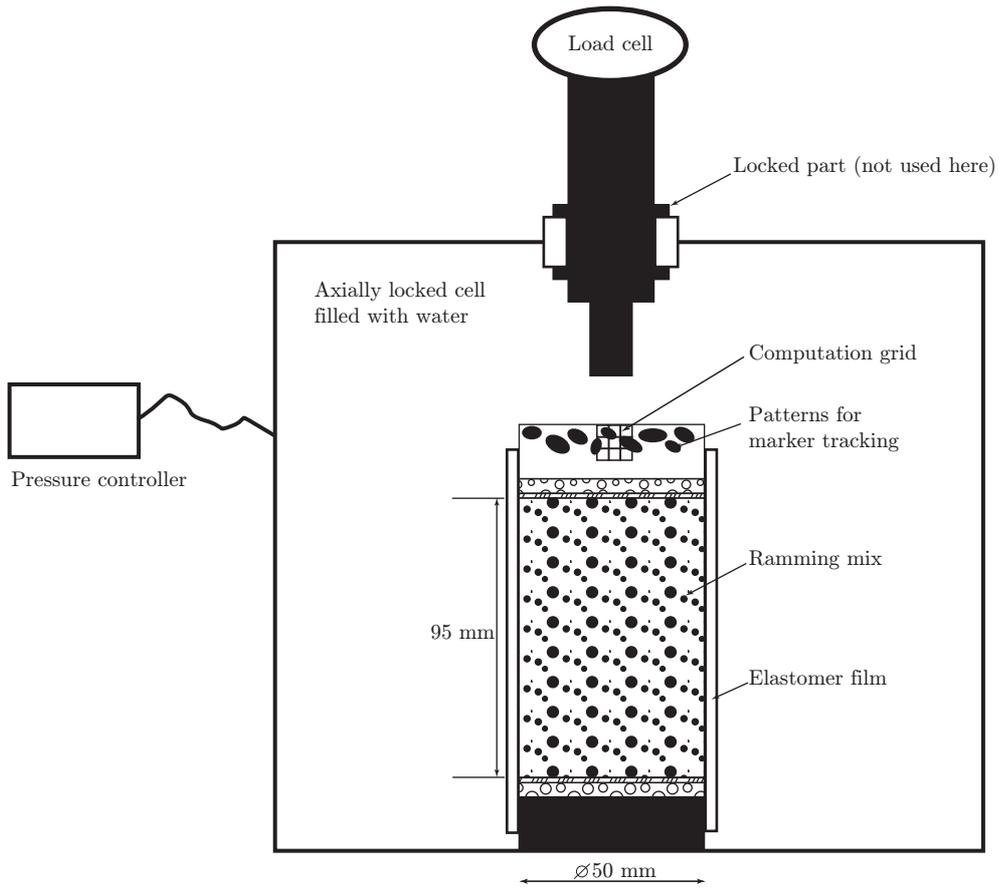


Figure 21: Hydrostatic compression test.

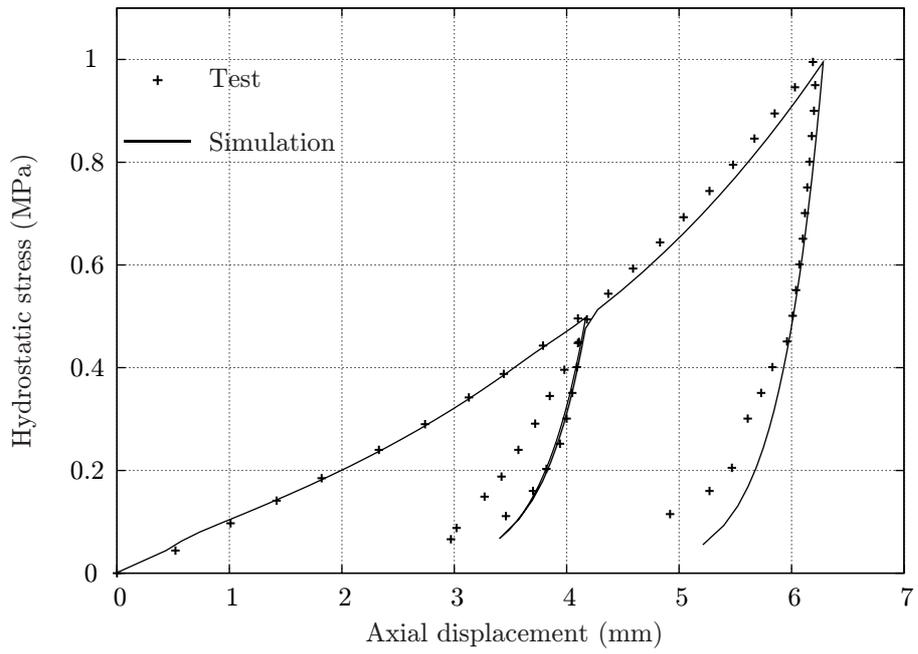


Figure 22: Hydrostatic compression test with loads/unloads.

Table 1: Hardening behaviour parameters.

Temperature ( $^{\circ}\text{C}$ )	20	50	80
$\kappa$	0.017	0.017	0.017
$\lambda_1$	0.12	0.12	0.12
$\lambda_2$	0.024	0.024	0.024
$e_1$	0.16	0.13	0.1

Table 2: Modified Cam-Clay model parameters.

Temperature (°C)	20	50	80
$\nu$		0.25	
$\kappa$		0.017	
$M$	1.18	1.1	1.1
$\lambda_1$		0.12	
$\lambda_2$		0.024	
$e_1$	0.16	0.13	0.1
$\beta$	1	0.56	0.21

Table 3: Thermal properties.

	Steel	Ramming mix
Density ( $\text{kg}\cdot\text{mm}^{-3}$ )	$7.85 \times 10^{-6}$	$[1.33 \times 10^{-6}; 1.73 \times 10^{-6}]$
Conductivity ( $\text{W}\cdot\text{mm}^{-1}\cdot\text{K}^{-1}$ )	0.047	0.025
Specific heat ( $\text{J}\cdot\text{kg}^{-1}\cdot\text{K}^{-1}$ )	450	700
Expansion ( $\text{K}^{-1}$ )	$1.2 \times 10^{-5}$	$3.3 \times 10^{-6}$